

Interactions between C-steel and blended-cement in concrete under radwaste repository conditions at 80 °C

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Abstract

Deep geological repository is widely considered as the preferred solution for the final disposal of high-level radioactive waste. Investigation representative of the Hungarian disposal concept was conducted using mock-up diffusion cells to study the chemical changes of S235JR carbon canister and CEM II/B PURAM concrete under anerobic and water saturated conditions at 80 °C. Micro-Raman, SEM-EDX, fluid and potentiometric analysis were performed over a period of 12 months. The discussion was supported by thermodynamic and reactive transport modeling. The findings revealed that a uniform corrosion process occurred, leading to rapid passivation of the C-steel with magnetite as the primary corrosion product. Modeling demonstrated that the increase in temperature to 80 °C and the chemical evolution of the concrete did not significantly affect the corrosion passivation process. Although the formation of Fe-siliceous hydrogarnets is thermodynamically possible at 80 °C, it did not jeopardize magnetite passivation. Overall, the results suggest that the containers would have a long lifespan under the studied disposal conditions.

Introduction

All states that engage in any kind of nuclear application must consider the management of radioactive waste and make sure it is handled in a safe manner regarding the level of radioactivity and complying with national/international regulations. There is a broad consensus that the preferred method of ensuring long term safety for high level radioactive waste (HLW) is isolation in a deep geological repository (DGR), which will provide passive multibarrier isolation of radioactive materials. Achieving this goal requires both natural geological barriers and engineered barrier system (EBS) with

complementary safety functions, creating a robust system to enhance confidence in the protection that will be provided [1, 2]. The EBS itself comprises a variety of sub-systems, such as the waste form (radioactive material immobilized in a host material), a corrosion resistant and mechanically stable container, a buffer/sealing system, and plugs. The EBS must be designed so that it will work with the natural barriers to meet the regulatory limits [3, 4]. The vitrified HLW form in a steel canister is specifically designed for long term durability in storage and disposal [5, 6]. The requirements for lifetime and integrity of the steel canister depend on the DGR concept and the geologic formation [7]. The radioactivity of the HLW will generate important heat power for several centuries to thousands of years depending on the HLW types, which will lead to maximal temperatures at the canister/buffer interface around 90°C [8]. After the sealing of the HLW disposal cell, oxygen will be rapidly consumed, and anoxic conditions will prevail [9].

The Hungarian radioactive waste management company is designing a DGR in the Boda Claystone Formation (BCF) in West-Mecsek (SW-Hungary). The interface between carbon steel and CEM II-based concrete is a key issue in the design of a disposal cell for vitrified HLW in argillaceous rock formations for the Hungarian national waste disposal program [10, 11]. The design relies on low carbon-steel (C-steel) containers containing the HLW encased in a prefabricated cylindrical concrete buffer material. The concrete is originated from the Public Limited Company for Radioactive Waste Management (PURAM), considered as the buffer material in the final disposal program in Hungary [12]. This concept presents many similarities with the super-container concept in Belgium and the Netherlands with C-steel overpack encased in cylindrical concrete buffer consisted of CEM I and limestone aggregates [13]. The present investigations will also help to optimize the Hungarian repository concept with the aim of ensuring its long-term safety.

C-steel is never in equilibrium with water even under anoxic conditions and will be subjected to several types of corrosion mechanisms, such as uniform corrosion and localized corrosion (e.g. pitting corrosion). The corrosion of C-steel in concrete has been widely investigated [14, 15], particularly focusing on aerated and rather low temperature in the framework of the durability of steel reinforced concrete in civil engineering. To our knowledge, the corrosion under elevated temperature and anoxic conditions representative of nuclear waste geological disposal has been much less studied in the literature. Recently, Chomat et al [16] have performed experiments with steel coupons embedded in blended (CEM V/A) cement pastes in water saturated and anaerobic conditions at 50°C for 3 years. A Fe-enriched layer formed directly at the surface of the steel which thickness did not increase with aging

time. The measured corrosion rates were typical of a passive corrosion mechanism. Smart and coauthors investigated the anaerobic corrosion of carbon steel wire in aqueous solutions representative of young CEM I porewater at 25 and 80 °C [17]. The corrosion rate was initially high but rapidly fell below 0.1 $\mu\text{m}/\text{y}$ probably due to magnetite passivation. Pally et al [18] have recently tested the anaerobic corrosion of metallic iron plates immersed in synthetic CEM I water at 80 °C. They demonstrated that a thin film of magnetite formed on the plates during the first immersion days followed by the formation of a compact layer of Fe-siliceous hydrogarnet (with passivation properties).

This study focuses on C-steel corrosion in contact with CEM II-based concrete at 80 °C to gain further information on the chemical-physical alteration of the steel/concrete interface for conditions representative of geologic disposal. Lab scale experiments were performed at a temperature of 80 °C, under water-saturated and anaerobic conditions during 12 months. A set of complementary characterization methods were applied to the liquid and solid phases with the support of geochemical modeling.

Materials and experiments

Concrete and steel compositions

The concrete originated from the National Radioactive Waste Repository in Bataapáti (Hungary). This underground repository ensures the final disposal of short-lived low- and intermediate-level solid or solidified radioactive waste of nuclear power plant origin [19]. The concrete was made of CEM II/B (18 wt.%), siliceous sand/gravel (54 wt.%), limestone flour (15 wt.%) and tap water (11 wt.%), as detailed in **Table 1**.

The steel container was made of untreated S235JR carbon steel. The chemical purity of the C-steel corresponded to ≤ 0.17 C, ≤ 0.3 Si, ≤ 1.4 Mn, 0.035 P, 0.035 S, ≤ 0.55 Cu, ≤ 0.012 N in wt.%.

Design of the cell experiments

The experimental set-up was prepared in triplicate as shown in **Fig. 1**. For each cell two Teflon containers were built to ensure water saturation during the experiments. The concrete was poured into the Teflon molds, placing the C-steel container in the center. The core of the cell was composed by a steel container with a height of 45 mm and diameter of 20.64 mm. The external Teflon container had a height of 160 mm, a diameter of 90 mm. The internal Teflon container had a height of 100 mm and a diameter of 50 mm. The internal Teflon container surface area was about 140 cm^2 and 5 holes with 0.7 mm diameter were randomly drilled through it to ensure saturation. The soaking water was made by

mixing 77 g of crushed concrete with 700 ml of MQ-water for three days. To exclude leakage, the mass of each experimental setup was inspected monthly. All three cells were closed to keep anaerobic conditions. A constant temperature of 80 ± 2 °C was imposed during the whole experiment. One of the experimental setups was opened for post-mortem characterization after 3, 7 and 12 months (hereafter referred as SC-3M, SC-7M, SC-12M with SC for Steel/Concrete). After disassembling the experimental setup, the C-steel containers were covered with adhering concrete making direct characterization of steel surface difficult. The corrosion interfaces were thus investigated on cross-sections.

Experimental results

Corrosion potential

Fig. 2 gives the relationship of corrosion potential with months of exposure. The evolutions of the corrosion potential measured on the three steel/concrete setups (3M, 7M, 12M) follow the same characteristics at all stages of experiments. The strongly negative values on the first two months are linked to the corrosion of the C-steel. Afterwards, the potentials decrease to reach small constant positive values that are assumed to be driven by the formation of a passivation layer. Film passivation in less than 50 days is remarkable for all three setups.

SEM/EDX analysis

Fig. 3 presents SEM images of the steel/concrete interfaces after 3, 7, 12 months experimental time. There is no indication of void or free space in between the concrete and the steel. The pouring of the concrete had led to tight interfaces since the beginning of the experiment. On each of the three samples the formation of 20-60 μm long Fe-oxide ingrowths were detected. These SEM results show that micro-cracks appeared even in a short term (3 months) and could have initiated changes of the carbon steel surface (**Fig. 3**). However, these ingrowths remained relatively short and did not exceed 100 μm in length after 7 and 12 months (**Fig. 4**).

Table 2 gives SEM-EDX analyses for minerals at the corrosion interfaces after 3 months (SC-3M) based on the points marked in **Fig. 3**. Corrosion products consisting of Fe – O (spectra 1-3) can be distinguished from the C-steel (spectra 4 and 5), which is primarily composed of Fe. **Table 3** gives the EDX analyses after 7 months with again a Fe – O signature typical of iron oxide corrosion products (spectra 2). But there is also a mixed signal composed of Ca – Al – Si – S and Fe oxide. **Table 4** provides results similar to those of **Table 2**.

Mineralogical phase identification

According to the results of the SEM-EDX, the corrosion process had an impact on the steel-concrete interfaces. With micro-Raman investigations mainly magnetite (Fe_3O_4) and hematite ($\alpha\text{-Fe}_2\text{O}_3$) can be identified as corrosion products after 3 months (SC-3M sample **Fig. 5**) and 7 months (SC-7M sample, **Fig. 6**). The corrosion protrusion is formed by magnetite, no other corrosion products could be identified by micro-Raman. After 12 months of experiments, no substantial change in the Raman spectra were observed (**Fig. 7**). Mainly magnetite (Fe_3O_4) and hematite ($\alpha\text{-Fe}_3\text{O}_4$) were still detected. During the whole exposure time, magnetite was identified as the main corrosion product. No iron-carbonates or iron-sulfides were identified.

Chemical analysis of aqueous solutions

The liquid phase (soaking water) was sampled from the external Teflon container after 3, 7 and 12 months. Ca, K, Mg, Na and Si concentrations were determined using inductively coupled plasma - optical emission spectrometry (ICP-OES), while ion chromatography (IC) was used for Cl^- and SO_4^{2-} (**Table 5**).

The Na and Ca concentrations progressively, but moderately, increased with time. The concentration of K and Si did not change significantly. Higher Mg concentration was measured after 12 months. The evolution of chloride and sulfate concentrations were less smooth, but the increase after 12 months was near 25% for chloride and 50% for sulfate.

Modeling results

Initial hydrochemistry and mineralogy of the concrete

The modeling was based on the recipe of the PURAM concrete detailed in **Table 1**. The oxide composition of the CEM II-B-S 42.5 N was taken from Lubloy et al [20]. The proportions of the CEM II-B components were fixed to 73.5 wt.% CEM I clinker, 24.5 wt.% blast furnace slag and 2 wt.% anhydrite (CaSO_4). The oxide compositions of the clinker and slag are detailed in **Table SI-1**. The hydration of the CEM I clinker was assumed to be completed, whereas only one half of the slag was assumed to have reacted. The limestone was considered as 100% CaCO_3 , the silica fume as 100% SiO_2 . The siliceous sand and gravel aggregates were assumed to be non-reactive during the time frame of the experiment and were not included in the modeling.

Table 6 gives an estimation of the initial reactive mineralogy of the concrete calculated at room temperature and 80 °C. The mineralogy is in line with the literature on clinker/slag cements [21, 22]. The rather high amount of Al in the slag leads to C-A-S-H phases instead of pure C-S-H ones. Al also

promotes the formation of monocarboaluminate (AFm phases) and ettringite (AFt phase). Mg is mostly found in hydrotalcite. In relation to the objective of this study, a sensitivity analysis was conducted to assess the impact of the ratio and degree of hydration on the mineralogy. The results indicated that, apart from the presence or absence of a small quantity of portlandite, no significant differences were observed. A higher degree of slag hydration (i.e. upper the 50 % assumed) may favor C-A-S-H at the expense of portlandite. The increase of temperature from 20 °C to 80 °C mainly results in the full dissolution of ettringite (and partial dissolution of monocarboaluminate) to form monosulfoaluminate (and calcite) in agreement with study of Lothenbach et al [23].

Table 7 provides for an estimation of the initial porewater chemistry of the concrete calculated at 80 °C in thermodynamic equilibrium with the reactive minerals of **Table 6**. The pH of young cement water is about 11.9 at 80 °C. This high pH value is driven by the dissolution of Na₂O and K₂O in water to form NaOH and KOH. The initial contents in Na₂O and K₂O were not accurately known. The modeling assumed that 50 % of these alkali hydroxides were sorbed onto the C-A-S-H phases. The increase of temperature decreases the pH and significantly increases sulfate concentration in the porewater. In the current modeling approach, the sorption of sulfate onto C-S-H was not taken into account. However, it is important to note that if this sorption process were considered, the dissolved sulfate concentration would be reduced by approximately 50 %. On the other hand, the presence of chlorides, which can exacerbate steel corrosion, was assumed to be fully dissolved in the porewater without being influenced by sorption or a mineral phase. At both temperatures, the initial chloride concentration was approximately 1200 mg/L.

Evolution of the aqueous chemistry with time

A reactive transport modeling had been performed in a second step based on the calculated initial chemistry and mineralogy of the concrete. The modeling grid took into account the full experimental cell set-up in 2D-cylindrical geometry (**Fig. SI-1** in supplementary information). The steel was not explicitly considered in the modeling that focused on the concrete/reservoir system. Steel is a non-porous material that only reacts through its surface and steel corrosion was found very low in the present study. The porosity and effective diffusion coefficient of the concrete was set to 10 % and 6×10^{-12} m²/s at 80 °C, respectively. These parameters had not been measured directly but estimated as a reasonable set according to the literature and provided a good fit to the concentrations of the (almost) no-reactive elements Na⁺ and Cl⁻ measured in the external Teflon container. The effective diffusion coefficient of the Teflon container, 3×10^{-13} m²/s, could only be estimated/fitted on the same data set. It is clear that the

5 drilled holes were sufficient to keep water saturation but induced a very low diffusive transfer. A sensitivity analysis on the effective diffusion coefficient of the container is given in **Fig. SI-2**.

Figs. 8 a-b show the evolution over 1 year of the concentration of the weakly reactive elements Na^+ and Cl^- that are only present as dissolved species in the model. Both concentrations are higher inside the concrete than in the soaking water. The gradient of concentration induces diffusion from concrete towards the external reservoir, which progressively leads to uniformized concentrations in the whole system. The good fit of the calculated concentrations after 1 year with the experimental data brings some confidence in the modeling. The decrease of the pH from 11.9 to about 11.2 inside the concrete (**Fig. 9**) is mainly due to the diffusion of the Na-K-OH plume towards the reservoir. Despite diffusion the pH remains quite high in the concrete and hence also at the interface with steel. The changes over time in the concentrations of Ca^{2+} and SO_4^{2-} (**Fig. 8 c-d**, reactive species) are influenced by both diffusion and the equilibrium with cement minerals. This equilibrium is, in turn, dependent on the pH value. **Table 7** gives the change in concentration of all the elements at the steel/concrete interface after 12 months. Such a relatively complex coupling between the chemical reactions and diffusion can only be handled with modeling. Again, the reasonably good agreement between the calculated and the experimental data gives credit to the model.

The main effect on the mineralogy of the concrete is due to the temperature increase up to 80 °C. Afterwards, the globally weak diffusive transfer does not induce any significant change in the mineral contents. The only-driven modification is the continuation of the transformation of monocarboaluminate to monosulfoaluminate and calcite at the boundary of the concrete with the reservoir but not around the steel/concrete interface.

Discussion on steel corrosion

Corrosion products at 80 °C

Geochemical and reactive transport modeling provides a reasonably accurate estimation of the mineralogy and chemical conditions at the steel interface, considering factors such as temperature rise and diffusion within the system. These geochemical parameters, which are difficult to measure directly, were effectively taken into account. Although the mineralogy remains relatively stable throughout the process, the chemistry of the porewater undergoes some changes, yet remains predominantly highly alkaline overall. Regarding the redox conditions induced by the concrete itself, it is currently uncertain. However, considering the anaerobic nature of the setup and the presence of slag, it is expected that

reducing conditions would be prevalent. Introducing metallic iron at trace level in the modeling of the initial geochemistry gives a calculated redox potential (Eh, SHE) ranging from -600 mV (with HS/SO₄ couple) to -750 mV (without HS/SO₄) at a pH of 11.9 and a temperature of 80 °C.

Fig. 10 shows some possible corrosion products that may form at the steel/concrete interface for the calculated chemical conditions. The Fe – O system points out the competition between goethite (Fe(III) oxyhydroxide) and magnetite (Fe(III)-Fe(II) oxide) according to the redox potential. It is worth mentioning that hematite will replace goethite in the same diagram if included in the system. If the system is further complexified by introducing Ca and Si, silica-hydrogarnet can prevail on magnetite at high alkaline pH but oxidizing to mildly reducing redox potential. Fe(II)-silicates corrosion products such as greenalite can only precipitate for pH lower than 10.5 at 80 °C and reducing conditions.

Magnetite was clearly identified as the main corrosion product by micro-Raman. The SEM-EDX analyses in Fe and O of corrosion products are also consistent with magnetite. Magnetite is usually the main product under anoxic corrosion of the carbon steel [5, 14, 16, 24]. No other corrosion products as Fe-silicate or Fe-sulfide were identified by this technique. Magnetite remained stable over the 12 months and did not convert to iron silicates and/or iron sulfides as it has been observed in cementitious/bentonite grout at lower pH [25]. Hematite has been identified in most crossed-sections, usually close to the steel surface and always in relatively low quantity compared to magnetite. It is possible that hematite was present since the beginning as mill scale [13], or else due to the transformation of early oxidic corrosion products during the preparation of the experimental set-ups and then heating to 80 °C.

Fe-siliceous hydrogarnet CFSH is one the most stable iron phase in hydrated cements [14, 22]. SEM-EDX spectrum 1 of **Table 4** indicates a mixed signature between concrete and iron oxide elemental composition. The analysis may correspond to a Fe-siliceous hydrogarnet, but it is most probably an intrusion of concrete in a crack within the steel. It seems that in the present system magnetite formed more quickly on the steel surface than CFSH formation controlled by Ca and Si diffusion from the concrete. Chomat et al. have also seen Fe-enriched layers at the concrete interface with steel [16]. Pally et al. have observed hydroandratite (CFSH) deposits on the iron plate at 80 °C but in their experimental set-up diffusion could have been facilitated by the direct contact of the steel with a synthetic solution [18].

Corrosion mechanism and rate

Microstructural characterization shows strong similarities in the corrosion patterns in 3, 7 and 12-months samples. Magnetite was found all along the steel surface although the formation of 20-60 μm

long Fe-oxide ingrowths were also identified. The literature states that the carbon steel surface is likely to be passivated due to the formation of a stable magnetite film [5, 16, 25]. This is in line with the short transient stage of negative corrosion potential measured in the three cells for 50 days at maximum. The occurrence of a uniform passive corrosion mechanism is, therefore, likely.

A mean corrosion rate was derived from observed thicknesses during SEM analysis and from the obtained corrosion potential curve (estimating that the altered layer was formed only in the first 50 days). In this study the estimated corrosion rate is in order of magnitude of 10 $\mu\text{m}/\text{y}$. Similar values of early corrosion rates have been found for CEM I and the corrosion rates usually decrease exponentially with time to reach rate smaller than 0.01 $\mu\text{m}/\text{y}$ within a year [17]. The long-term corrosion rate could not be determined in the present experiments but was clearly much lower than 10 $\mu\text{m}/\text{y}$. Temperature increase seems to be inoperative for the long-term corrosion rates [17].

There was no trace of any localized corrosion mechanisms. The moderated aqueous concentrations in chloride (500 – 1000 mg/L) had no effect, in agreement with that did not find any consequences of high concentrations of chloride on the corrosion of steel under highly alkaline anoxic conditions [17].

Conclusions

The current experimental setup examined the combined chemical evolution of two major components, namely the waste canister and the concrete buffer, used in deep geologic disposals at a temperature of 80 °C. The materials chosen for analysis, S235JR carbon steel and CEM II/B based concrete, are commonly used as reference materials in Hungarian disposal concepts. However, the findings are applicable to other disposal systems and engineering barriers as well. During the experiments, it was observed that a uniform corrosion process occurred, resulting in the rapid passivation of the C-steel cylinders. Throughout the entire 12-month duration of the experiment, magnetite was identified as the primary corrosion product, even after only 3 months of exposure. Modeling revealed that the increase in temperature and lateral diffusion of the concrete porewater led to a slight decrease in pH and chloride concentrations, as well as an increase in sulfate concentration due to the dissolution of ettringite. However, these chemical changes did not significantly affect the passivation corrosion process. Although the formation of Fe-siliceous hydrogarnets was thermodynamically possible at 80 °C, it did not hinder the formation of magnetite. Overall, these results indicate that the containers would have a long lifespan under the studied disposal conditions.

Methods

Corrosion potential measurements

The corrosion potential was monitored using a platinum wire that has been embedded within the concrete and C-steel wire (also S235JR) spot welded on the container. Measurements were taken at 20-second intervals. Initially, the potential between the platinum wire and a calomel electrode was checked using a Metrohm Autolab PGStat204 potentiostat. The measurements were conducted in the soaking water under specific conditions, namely at pH 11.9 and a temperature of 22.8 °C. To ensure accurate interpretation and comparison of the results, the obtained corrosion potential was then adjusted using an offset correction. This correction was applied to align the results with the standard hydrogen electrode (SHE) scale. By making this adjustment, the corrosion potential values can be properly referenced and analyzed in relation to other electrochemical systems or experiments conducted using the SHE scale.

Scanning Electron Microscopy measurements

The composition and nature of alteration products formed on the steel and within the concrete were investigated by scanning electron microscopy/energy dispersive X-ray spectroscopy (SEM/EDX). The measurements were performed at 20 kV, 1.6 nA, using a Thermo Scientific Scios2 dual beam microscope, Oxford X-maxn 20 SDD EDX [26].

Micro-Raman spectroscopy investigations

Micro-Raman investigations were performed to better identify the corrosion products. The analyses were carried out using a HORIBA JobinYvon LabRAM HR 800 Raman microspectrometer. A frequency doubled Nd-YAG green laser with a 532 nm excitation wavelength was used to illuminate the samples, displaying ~0.2 mW on the sample surface. An OLYMPUS 50× (N.A. = 0.6) and 100× (N.A. = 0.9) objective was used to focus the laser. For Raman mapping, 100 µm confocal hole, 600 grooves/mm optical grating, 4-10 s cumulated exposition time were used. The spectral resolution of measurements was 3.0 cm⁻¹. The step size of the Raman maps varied between 1 and 0.5 µm for magnifications 50× and 100×, respectively [26].

Inductively Coupled Plasma Optical Emission Spectroscopy and Ion Chromatography

ICP-OES measurements were carried out on a Perkin Elmer Avio 200 instrument. The leachates were filtered through a cellulose acetate membrane (dp > 220 nm) and then were acidified with 2 m/m% cc.

HNO₃. All elements were tested in radial view using 1 ppm of Y as internal standard and a calibration of 4 points was applied for all measured elements (0.01, 0.1, 1 and 10 ppm). The power of the generator was set to 1200 W during the measurements and the use of plasma argon was 12 l/min. IC analyses were performed on Thermo Scientific Dionex Aquion equipment with 4.5 mM Na₂CO₃/0.8 mM NaHCO₃ eluent composition and at a flowrate of 0.25 mL/min. The separation of the components was carried out by a Dionex IonPac AS23 2 mm x250 mm analytical column coupled with a Dionex AERS 500 Carbonate electrochemical suppressor.

Modeling approach and database

The reactive transport code HYTEC was used to model the initial geochemistry of the concrete as well as its evolution with temperature and time [27]. The objective was to obtain a reliable estimation of the chemical conditions within the system, specifically focusing on the interface between the steel and concrete. To the best of our knowledge this study represents the first attempt to model the CEM-II based concrete, which is relevant to Hungarian radwaste management. Additionally, phase stability diagrams (Eh-pH diagrams) were calculated with CHESS, the geochemical module of HYTEC. All calculations were performed at thermodynamic equilibrium. The Thermoddem database [28, version 12-2019] was selected which offers accurate thermodynamic constants at both 20 and 80 °C. This comprehensive database encompasses a wide range of minerals that are pertinent to cement phases and corrosion products. To account for activity corrections implementation of the truncated-Davies model was considered. Furthermore, the formation constants of the Fe-siliceous hydrogarnet (C3FS0.84H4.32) were incorporated into the database [29].

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Author contributions

MF: Conceptualization, Methodology, Funding acquisition, Investigation, Writing-original draft. OC: Characterization (ICP-OES and IC analysis, Corrosion measurements, Chemical analyses) IT: Characterization (SEM, μ -Raman spectroscopy) LdW: Geochemical modelling, conceptualization, writing-editing. All: Writing-review & editing.

Data availability

The data related to the present study can be obtained from the corresponding author M. Fábíán (fabian.margit@ek-cer.hu) upon personal request.

Competing interests

The authors declare no competing interests.

Ethics declarations

Not applicable.

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Figure legends

Fig. 1 a, b Cross-section (a) and photograph (b) of the experimental setup in triplicate. Photograph courtesy of Margit Fabian.

Fig. 2 Evolution with time of the measured corrosion potentials on the standard hydrogen scale for the three cells at 80 °C.

Fig. 3 a, b SEM micrograph showing micro-cracks initiated in the steel in contact with concrete after 3 months (SC-3M sample) (a) and the corresponding elemental mapping of Fe, O, Si, Ca and Mn (b).

Fig. 4 a,b SEM micrograph of the steel-concrete interface after 7 months (a) and 12 months (b).

Fig. 5 Micro-Raman mapping and characteristic spectra obtained after 3 months (SC-3M).

Fig. 6 Micro-Raman mapping and characteristic spectra obtained after 7 months (SC-7M).

Fig. 7 Micro-Raman mapping and characteristic spectra obtained after 12 months (SC-7M).

Fig. 8 a-d Modeling of the time-dependent evolution of the concrete porewater chemistry (in red) and the reservoir chemistry (in blue). For the purpose of comparison, experimental data from the reservoir are included as blue dots.

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Fig. 10 a,b Phase stability diagrams of possible corrosion products of C-steel in slightly to strongly alkaline chemical environments (CHESS/HYTEC code, Thermoddem database, Section *Modeling approach and database*); activity $\text{Fe}^{2+} = 5 \times 10^{-5}$, activity $\text{Ca}^{2+} = 5 \times 10^{-4}$, total $\text{H}_4\text{SiO}_4 = 1 \times 10^{-5}$, temperature = 80°C; the grey zones stand for likely pH – Eh conditions in the present steel/concrete system.

Tables

Table 1. Composition of concrete.

| Materials | | Density [kg/dm³] | Mass [kg/m³] | Volume [L/m³] |
|--------------------|-------------------|--|------------------------------------|-------------------------------------|
| Main components | | | | |
| Aggregates | sand | 2.7 | 920 | 347 |
| | gravel | 2.7 | 230 | 87 |
| Cement | CEM II/B-S 42.5 N | 3.1 | 385 | 126 |
| Limestone flour | limestone flour | 2.7 | 315 | 117 |
| Water | tap water | 1.0 | 231 | 231 |
| Minor components | | | | |
| Fiberglass | | 2.6 | 1 | 0.5 |
| Admixtures [30] | Sika ViscoCrete | 1.1 | 5 | 5 |
| | Sika Control-40 | 1.0 | 10 | 10 |
| | Sika LPS A-94 | 1.0 | 1 | 1 |
| | SikaFume | 2.2 | 26 | 11 |
| Total | | | 2 124 | 935 |

Table 2. SEM-EDX analyses of the corrosion products and the steel for the SC-3M sample (the locations of the spectra are given in Fig. 3 a).

| [at. %] | Spec 1 corr. prod. | Spec 2 corr. prod. | Spec 3 corr. prod. | Spec 4 C-steel | Spec 5 C-steel |
|---------|-----------------------|-----------------------|-----------------------|-------------------|-------------------|
| O | 47.09 | 44.01 | 47.64 | - | - |
| Si | 1.67 | 0.18 | 0.47 | 0.65 | 0.97 |
| Ca | 0.16 | 0.15 | 0.11 | - | - |
| Mn | 0.36 | 0.28 | 0.21 | 0.59 | 0.61 |
| Fe | 50.72 | 55.38 | 51.57 | 98.76 | 98.42 |
| Total | 100.00 | 100.00 | 100.00 | 100.00 | 100.00 |

Table 3. SEM-EDX analyses of the corrosion products and the steel for the SC-7M sample (see Fig. 4 a).

| [at. %] | Spec 1 concrete/corr. prod. | Spec 2 corr. prod. |
|---------|--------------------------------|-----------------------|
| O | 56.45 | 46.84 |
| Mg | 3.43 | - |
| Al | 2.41 | - |
| Si | 12.10 | 0.34 |
| P | 0.06 | - |
| S | 0.51 | - |
| Cl | 0.05 | - |
| K | 0.08 | - |
| Ca | 3.59 | 0.14 |
| Ti | 0.06 | - |
| Mn | 0.15 | 0.37 |
| Fe | 21.13 | 52.31 |
| Total | 100.00 | 100.00 |

Table 4. SEM-EDX analyses of the corrosion products and the steel for the SC-12M sample (see Fig. 4 b).

| [at. %] | Spec 1 C-steel | Spec 2 corr. prod. | Spec 3 corr. prod. |
|---------|-------------------|-----------------------|-----------------------|
| O | - | 51.46 | 58.26 |
| Si | 0.46 | - | - |
| Ca | - | 0.08 | 0.12 |
| Fe | 99.54 | 48.46 | 41.62 |
| Total | 100.00 | 100.00 | 100.00 |

Table 5. Evolution of the chemistry of the aqueous solution sampled in the external Teflon container (Fig. 1 b).

| Concentration [mg/L] | Ca | K | Mg | Na | Si | Fe | Al | Cl ⁻ | SO ₄ ²⁻ |
|-------------------------|-----|----|----|-----|----|------|-----|-----------------|-------------------------------|
| Soaking water | 97 | 24 | 2 | 219 | 43 | 0.4 | 0.8 | 505 | 100 |
| 3 months | - | - | - | - | - | - | - | 449 | 157 |
| 7 months | 117 | 19 | 1 | 238 | 44 | <DL* | <DL | 505 | 115 |
| 12 months | 135 | 26 | 18 | 299 | 47 | <DL | <DL | 618 | 145 |

* DL stands for detection limit.

Table 6. Calculated initial amounts of reactive hydrated phases of the concrete.

| Solid phases * [kg/dm ³ of concrete] | T = 20 °C | T = 80 °C |
|--|-----------|-----------|
| C1.6-A0.01-S-H | 0.357 | 0.357 |
| Portlandite** | 0.044 | 0.044 |
| Ettringite | 0.031 | 0.000 |
| Monocarboaluminate | 0.054 | 0.028 |
| Monosulfoaluminate | 0.000 | 0.044 |
| Hydrotalcite | 0.025 | 0.025 |
| Calcite | 0.350 | 0.355 |

* The non-reactive sand and gravel aggregates were not assumed to be reactive phases.

** Portlandite occurrence depends upon the degree of slag reactivity.

Table 7. Calculated pore water chemistry of the concrete at 20 °C and 80 °C.

| Total concentration [mg/L] | T = 20 °C initial | T = 80 °C initial | T = 80 °C Steel interface, 12 months |
|---------------------------------------|------------------------------|------------------------------|---|
| pH | 13.6 | 11.9 | 11.25 |
| Na ⁺ | 4350 | 4350 | 540 |
| K ⁺ | 7450 (750 *) | 7450 (750 *) | 605 (60) |
| Ca ²⁺ | 35 | 29 | 149 |
| Mg ²⁺ | 0 | 0 | 0 |
| Al ³⁺ | 1 | 145 | 28 |
| Cl ⁻ | 1200 | 1200 | 563 |
| HCO ₃ ⁻ | 14 | 27 | 1 |
| SO ₄ ²⁻ | 256 | 3300 | 66 |
| H ₄ SiO ₄ | 5 | 9 | 1 |

* K⁺ concentration required to fit the data of the Teflon container.

Figures

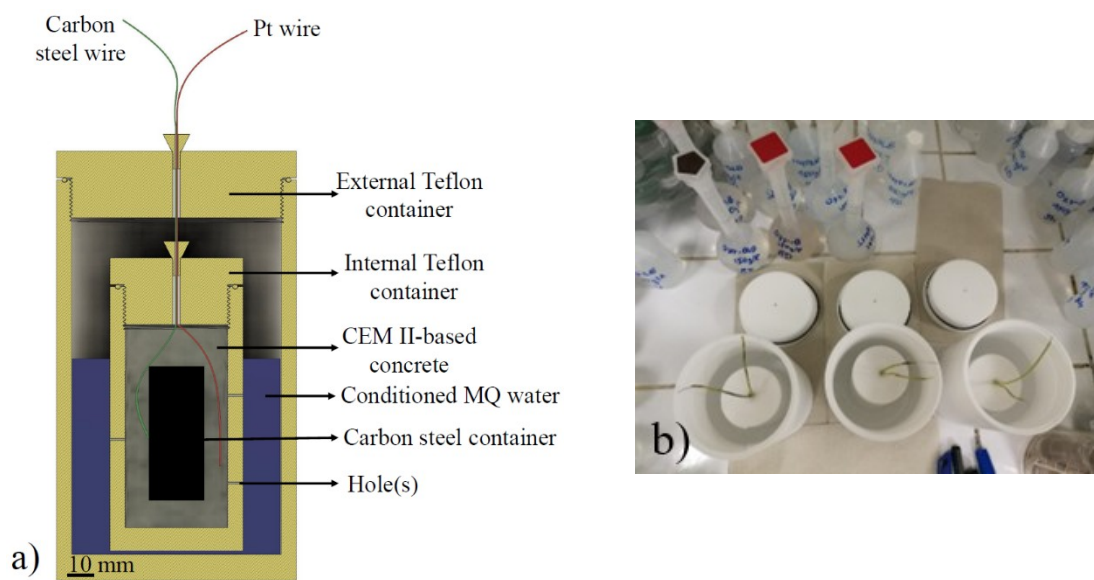


Fig. 1 a, b Cross-section (a) and photograph (b) of the experimental setup in triplicate. Photograph courtesy of Margit Fabian.

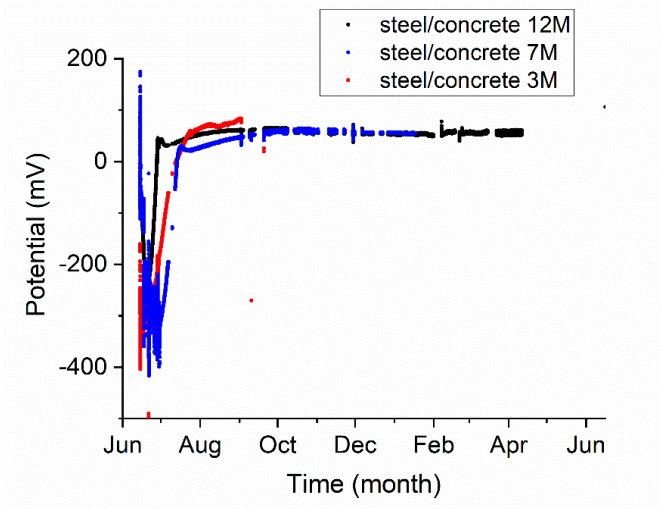
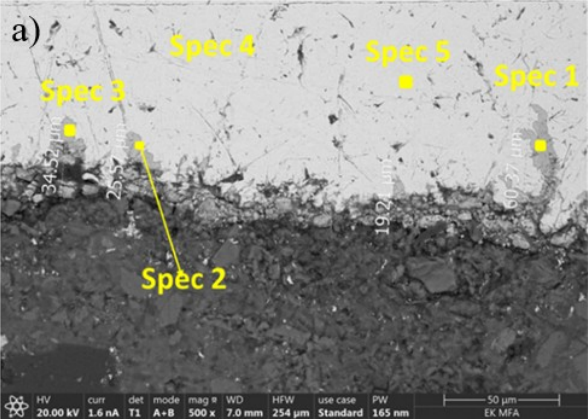


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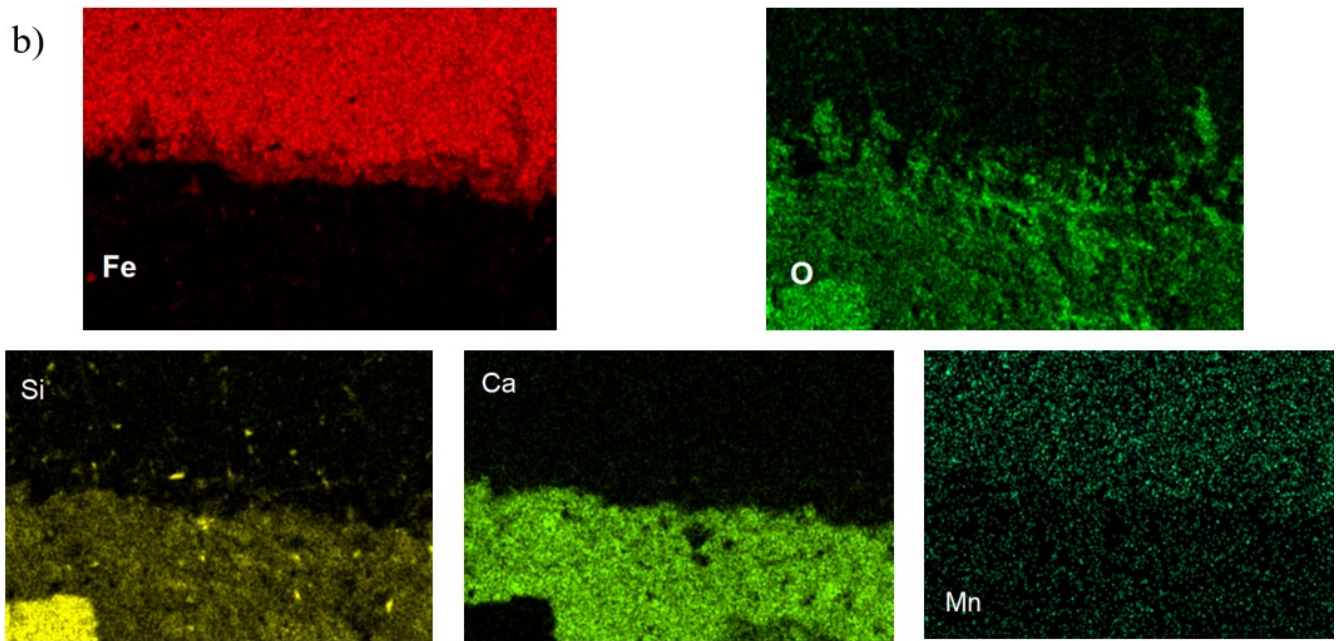


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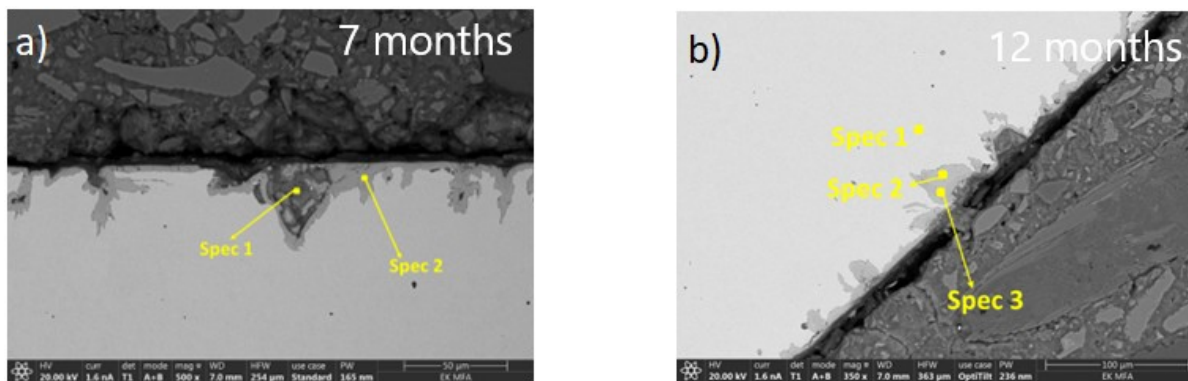


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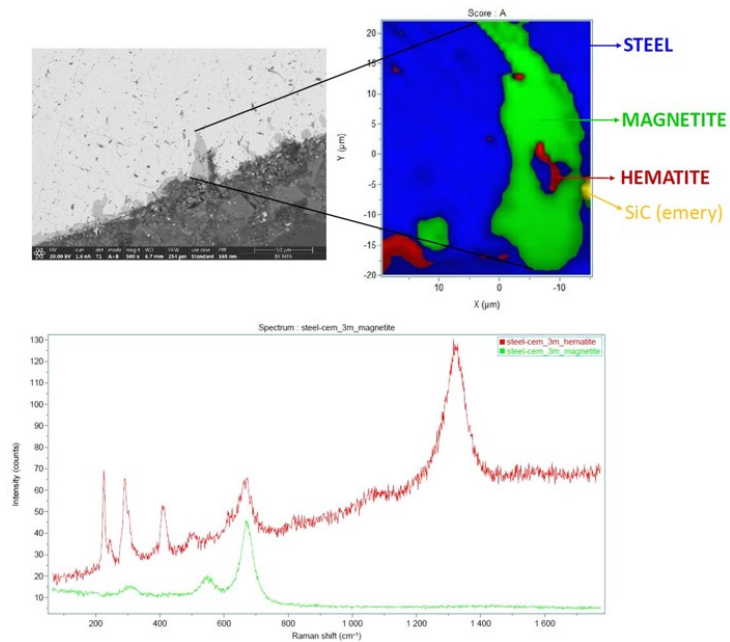


Fig. 5 Micro-Raman mapping and characteristic spectra obtained after 3 months (SC-3M).

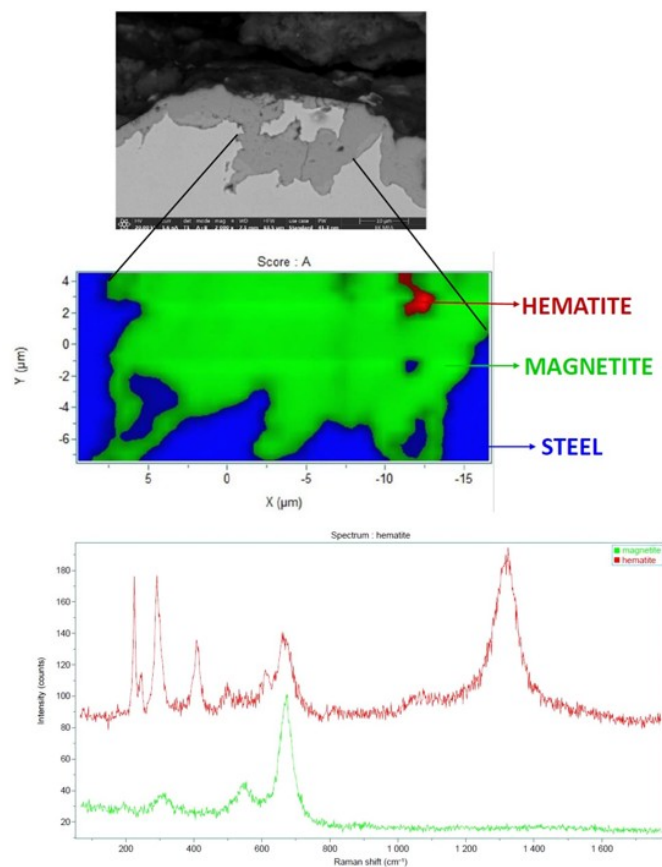


Fig. 6 Micro-Raman mapping and characteristic spectra obtained after 7 months (SC-7M).

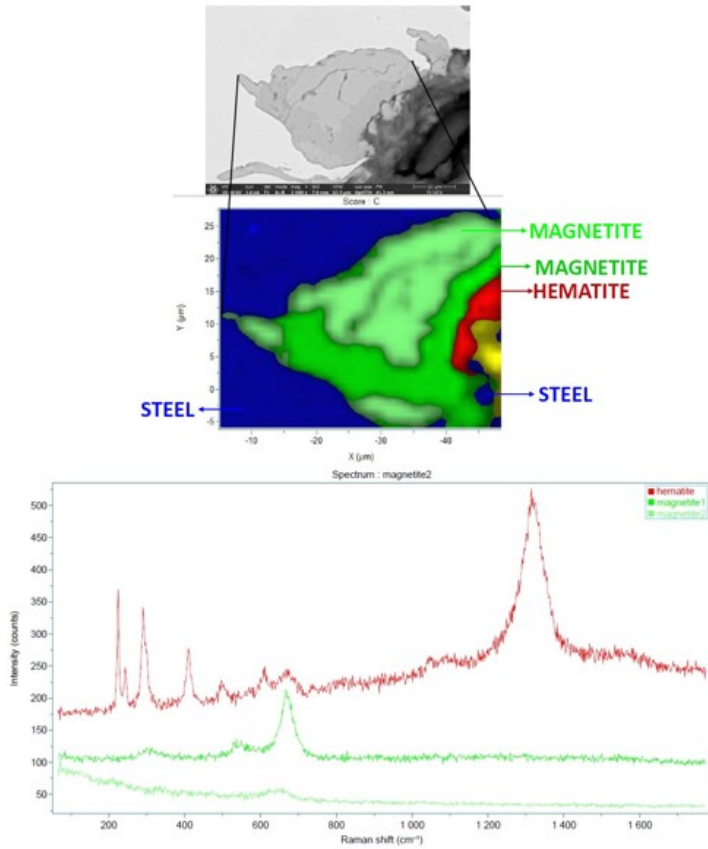
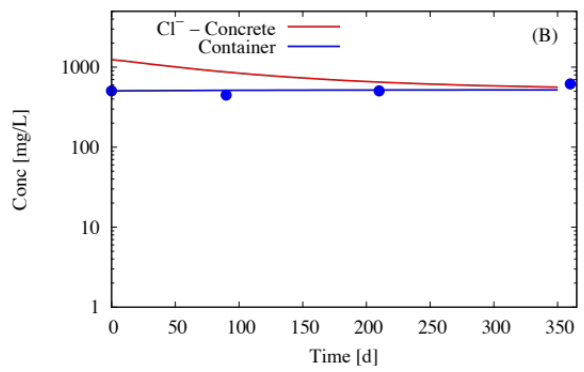
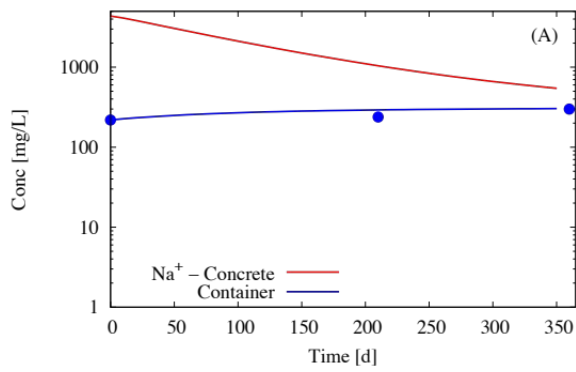


Fig. 7 Micro-Raman mapping and characteristic spectra obtained after 12 months (SC-12M).



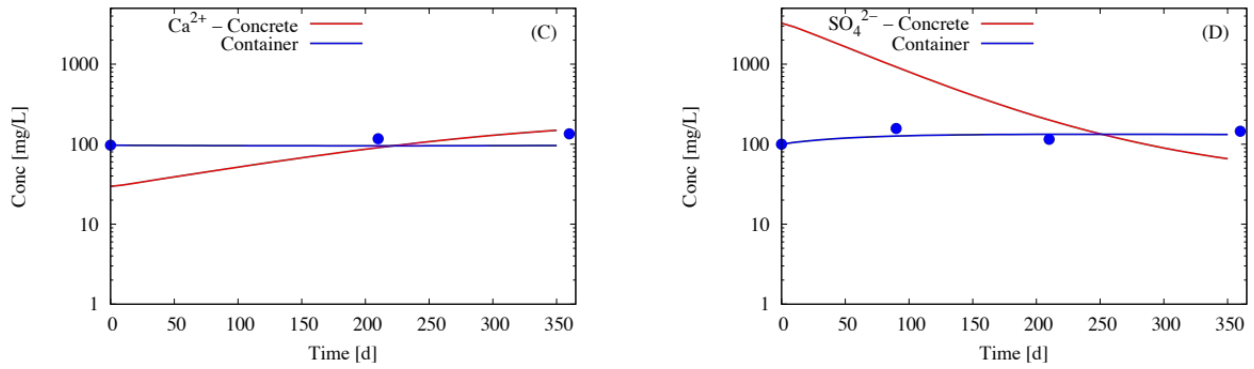


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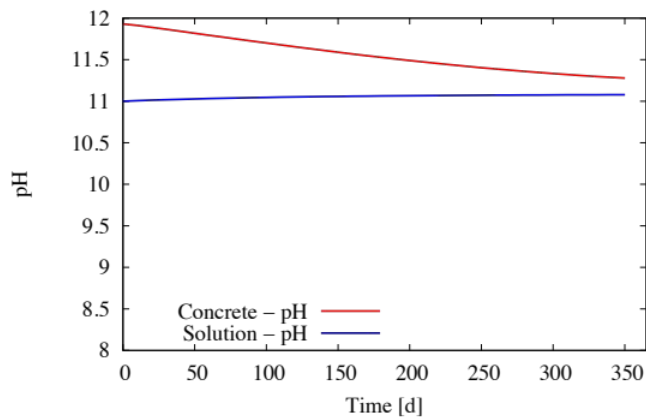


Fig. 9 Modeling of the evolution with time of the pH of the porewater (in red) and the reservoir (in blue).

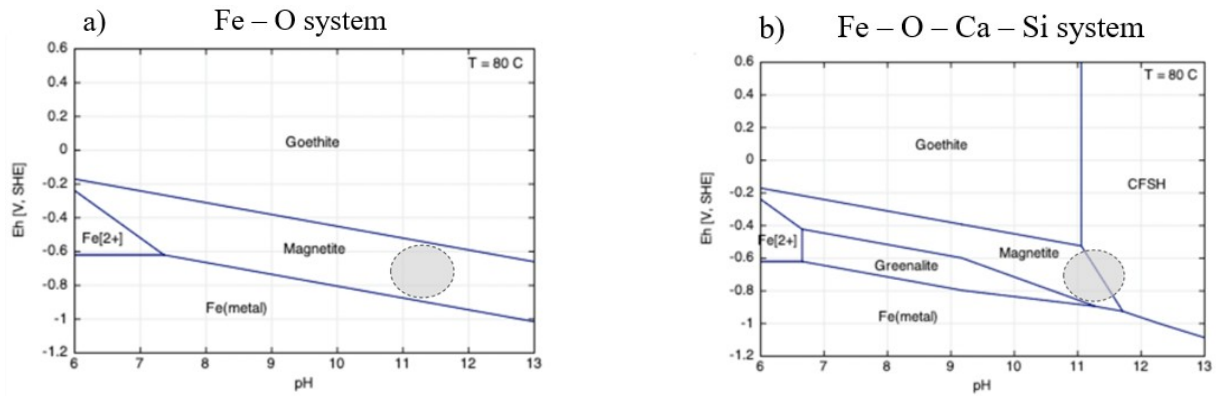


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