

INVESTIGATION OF SEEPAGE EFFECT ON RIVER DIKE'S STABILITY UNDER STEADY STATE AND TRANSIENT CONDITIONS

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Abstract: World experiences reveal that catastrophic floods are posing a serious threat that comes not only from them as extreme events but also as the result of adaptation measures uncertainty, (i.e. dikes). In particularly old dikes constructed earliest at the north part of Albania, during the last floods demonstrated the high uncertainty and weak spots. In this paper, the significance of the seepage investigation and stability analysis are discussed. As a case study, parts of new dikes constructed last years along the Buna River located in north part of Albania are investigated. Filling materials for these dikes are represented mostly from silt and clay. Finite element method is used to perform seepage analysis while general limit equilibrium method is used to perform slope stability analysis. Both, seepage and slope stability analyses are done for three different scenarios: steady state, rapid filling, and rapid drawdown. Finally, it is concluded that silt-clay material used in these dike structure is posing serious uncertainty during seepage phenomenon by threatening the stability of entire dike structure especially during the transient condition (rapid filling and drawdown).

Keywords: Dike, Seepage, Slope Stability, Rapid Filling, Rapid Drawdown

1. Introduction

It is evident that considerable numbers of inhabitants are located closer to lakes, shoreline, and rivers valley. In one point of view, establishment of high percentage of inhabitants to the aforementioned locations has many opportunities but on the other hand, these locations are vulnerable from different kinds of threat posed from extreme events as increase of sea level, high magnitude of wave and flooding events. Periodically floods pose a serious threat to human life, valuable property and city

infrastructure, [1], [2]. Construction of the adaptation and defenses systems is dating very earlier in human being history. Adaptation systems are represented by structures as dikes or embankment, which has two main purposes; to retain and diverting certain amount of water that comes especially during flood events, [3], [4]. The first measures to be adopted against these extreme events and to avoid drastic damages is related to better conception of adaptation structures by considering different records from flood events, [5]. In the past, construction of these kinds of structures generally has been carried out by using simple procedures and equipment. In this context the older dikes or embankment structures in generally, are presenting high index of risk related to failure, especially during flood events, [6], [7]. The main factors that influence a dike failure depends mostly from: type of materials (heterogeneity), used as filling material, not sufficient compaction during the construction phase, which leads to the softening and loosening of filling materials.

Adaptation measures against natural hazards [8] or those initiated by man-made are one of the main challenges, going to the past it is obvious that a serious effort is taken from people after they are affected especially from flood disasters. During the last decades, floods probability of occurrence has increased in terms of the return period (i.e. probability exceedance). So having sufficient information on earth structures and specifically on dike performance is very important in order to have clear picture about dike stability against different factor that cause failure of structure itself, [9], [10]. In this paper, seepage effects on dike structures stability during steady state and transient conditions are discussed. Additional factors that cause instability or in worse case total failure of earthen dike structure in generally are: surface erosion, piping and seepage. From these three factors, erosion can be consider as less threat factor that may cause failure of dikes since it occurred at the outer part of the structure, so it can be controlled. On the other hand, piping, which is considered as one of the main factors that cause failure of earth structures all over the world, occurs due to the migration of small soil particles into the coarse materials. This phenomenon takes place through the dam or dike body, or under the structure foundation, [11], [12].

While the phenomenon of seepage that is discussed mostly in this paper represent the uncontrolled saturation of filling materials inside the dike or earth structures in generally. In this context, the stability of dikes is assessed for steady state and transient condition. Steady state condition consists into the investigation of seepage effect on dike stability when water level does not change. While transient condition is regarding with phenomena known as rapid filling or rapid drawdown, which occurs during the sudden change of water level (increments and decrements), which modifies flow condition and effect directly soil properties inside the dike structure. Numerical modeling based on finite element method is used to analyze the seepage for respective conditions (i.e. steady state and transient). Finally, achieved results are compared for both conditions and some main conclusion and recommendations are addressed to the protection of dikes or embankment structures.

2. Structural failures

2.1. Dike breaching mechanisms

Failures of an earth structure involve the separation (rupture) of the embankment material or its foundation depending on geotechnical conditions of the basement. This type of failure is more prominent in large dams or embankment structures. However, it is not exclusive to only large dams; similar occurrences happen on small earthen embankments or dikes, [13], [14]. It is noticed that the failure of earth structures may be caused due to different factor as construction activities, rainfall infiltration, also many failures occur in natural soil slopes and excavated slopes. In many cases, stability problems of embankments structures and dikes occur during the rapid filling of reservoir or waterways and rapid drawdown, [15].

According to 'Voorschrift Toetsen op Veiligheid'; National Protocol for Safety Assessment of dikes in Netherlands [16], safety assessment of dikes should be done by considering one of the 9 potential failure mechanisms: slide circle inside slope, settlement, slide circle outside slope, piping, wave overtopping, erosion outside slope, micro instability, softening and erosion foreland. Generally, natural soils are characterized from high variability and heterogeneity, these characteristics as mentioned above has a significant indication on physical-mechanical properties of the soil. One of the main sources of heterogeneity is inherent spatial soil variability due to the different depositional conditions and different loading histories, [17], [18], [19].

Breach mechanism is classified as following: hydraulic failure, geo-hydraulic failure and global static failure, [20]. Hydraulic failure generally consists on overtopping [21] and wave scour on the dike structure, while geo-hydraulic failure is due to the seepage process through the dike core or foundation, which leads into the erosion process associated with transportation of significant materials amount. Whereas concerning to the type of breach mechanisms, if two aforementioned types are induced mainly from pressure forces as ice, water, and wind waves, while the third type of breach mechanisms (i.e. global static failure) is due to gravity and pressure forces occurring as local or total failure of the dike structure.

2.2. Seepage phenomenon

Analyzing seepage phenomenon occurred in: earth irrigation system, storages, ponds and other earth structure as dikes or earth dams is very important task to be carry out in order to avoid any threat that may come as result of any possible failure, [22]. Earliest, attention has been given generally on estimation of seepage loose regarding especially to channel system and storage dam, while nowadays is given attention both amount of water loose and impact on environmental degradation, [23]. Identification and estimation of seepage phenomenon are very important issue for design and construction in particularly for the earth dams and dike structures, [24]. In order to decrease the path of flow line, which represents the seepage phenomenon through the earth structure, designers proposing to use one of the following measures: impervious core, upstream clays and blanket or cut-off trench. Still, even significant advancement achieved in the field of geotechnical engineering, excessive seepage under or through the structure,

threatens seriously the integrity of the structure, [25], [26]. In unconfined zone regarding the earth structures, seepage origin is linked also with the free surface of the structure. Free surface at the mean time play two different roles: it behaves as path of streamline and as boundary condition within the seepage network by hindering the procedure of calculation, [27], [28]. As mentioned above during flood events, seepage phenomena are one of the main factors that induce failure of dike structures. The instantaneous increase and decrease of water level at the river dike during the floods, induce a typical unstable seepage process, moreover in these conditions a significant deformation of structure can occur, [29].

3. Problem statement

Floods are natural phenomena, which happening very often especially in the north-west part of Albania [30], (*Fig. 1*). During floods, water occupy for hours in case of flash floods, or days in case of long-term floods that occurs generally during the winter; when rainfall period is longer or spring as result of snowmelt, [31].

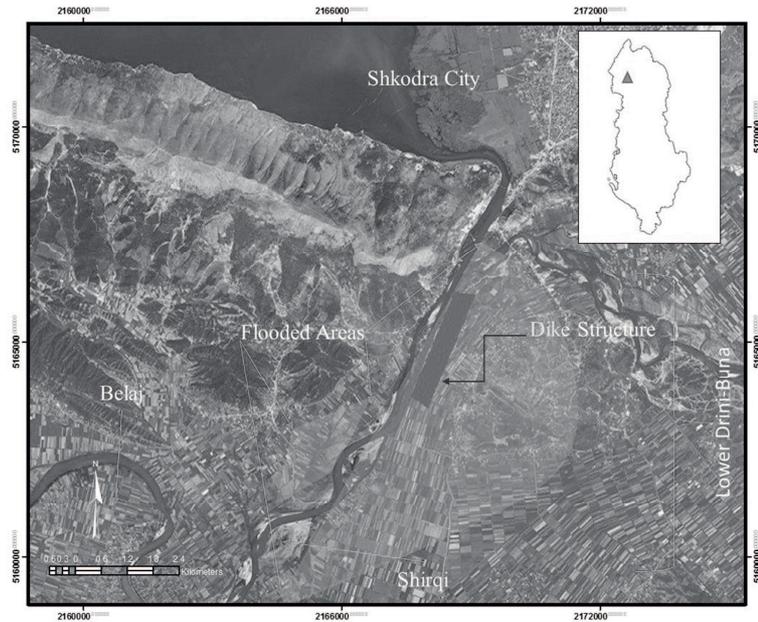


Fig. 1. Flood plan map of Buna River in Shkodra city and surrounding areas, located at north-west of Albania

An unacceptable high risk of flood characterized the land of the Lower Drini-Buna River basin, this is due to significant geological changes that happen last 150 years ago. Along the Buna River flow, adaptation measures are constructing continuously, especially after floods of winter 2010. Despite new protection structures constructed

after winter 2010, in addition, there is done reinforcement of the existing dikes. The elevation of the existing dikes on the left bank of the Buna is placed in accordance with an international agreement between Albania and Montenegro. This level provides protection against the calculated flow discharge 2% exceedance probability (i.e. 1-50-years), [32]. Even if flow substantially in excess of the 100-year design flood arrives at Shirqi, a weir is designed such that the excess flow, above the 50-year discharge, is diverted out of the river and across Trushi Field towards Murtemza. The Shirqi to Belajdikes and the Pentari to Pulaj dikes, when rehabilitated to internationally agree levels, are protecting the south-west of the study area, including the town of Velipoja for excess flow, 1 to 50-year exceedance probability. Since in the Buna River, continuously is occurring rapid filling and drawdown, this process leads to seepage phenomenon even during the construction phase. In addition, this may affect the stability of protection structures, especially earth structures (i.e. dikes). In this case as mentioned above, seepage and stability of dike analysis is conducted by considering different scenarios: steady state, rapid filling, and rapid drawdown.

4. Materials and methods

4.1. Materials

During the construction phase of dikes placed along the Buna River, raw materials used for construction the dikes structures are represented mostly from silt-clay. Detailed data information about materials shown below (*Table I*) are obtained after in situ testing.

Table I

Materials properties used for dike construction

Type of Construction Materials	Parameters		Quantity
	Notations	Symbols	
Silt-Clay	Hydraulic Conductivity (m/h)	$K_{saturation}$	0.0036
	Volumetric Water Content (m^3/m^3)	θ_w	0.08
	Saturated Volumetric Water Content (m^3/m^3)	θ_s	0.55
	Residual Water Content (m^3/m^3)	θ_r	0.18
	Minimum Suction (kPa)	S_{min}	0.01
	Maximum Suction (kPa)	S_{max}	1000
	Unit Weight (kN/m^3)	γ	17
	Cohesion (kPa)	C	15
	Friction Angel (degree)	ϕ'	20
	Curve Fitting Parameter	n	1.3
Curve Fitting Parameter (cm^{-1})	α	0.78	

Volumetric water content is predicted by using Van Genuchten method as given in Eq. (1):

$$\theta_w = \theta_r + \frac{\theta_s + \theta_r}{\left[1 + \left(\frac{\Psi}{a}\right)^n\right]^m}, \quad (1)$$

where, θ_w is the volumetric water content; θ_s is the saturated volumetric water content; θ_r is the residual water content; Ψ is the negative pore-water pressure; and a, n, m are curve fitting parameters. According to Van Genuchten method, curve-fitting parameters are estimated based on volumetric water content of soil. Therefore, the best point to estimate and evaluate the curve fitting parameters is the middle point between residuals and saturated water content, [33], [34].

The slope of curve can be estimated by using Eq. (2) as following:

$$S_p = \frac{1}{\theta_s - \theta_r} + \frac{d\theta_p}{d(\log \Psi_p)}, \quad (2)$$

where, θ_p is the volumetric water content at middle point; Ψ_p is the matric suction at middle point. While parameters a and m are estimated as following by Eq. (3) and Eq. (4):

$$m = 1 - \exp(-0.8S_p), \quad (3)$$

$$a = \frac{1}{\Psi} (2^{1/2} - 1)^{1-m}. \quad (4)$$

Based on Van Genuchten method, hydraulic conductivity of soil can be estimated by Eq. (5);

$$k_w = k_s \frac{\left[1 - \left(a \Psi^{n-1} (1 + a \Psi^n)^{-m}\right)\right]}{\left(1 + a \Psi^n\right)^{m/2}}, \quad (5)$$

where, k_w is the hydraulic conductivity; and k_s is the saturated hydraulic conductivity. Relation between matric suction, volumetric water content and conductivity are presented below, (Fig. 2). Within the increase of matric suction, volume water content

and conductivity are decreasing because of compaction and consolidation process, which leads to increase of soil density, [35].

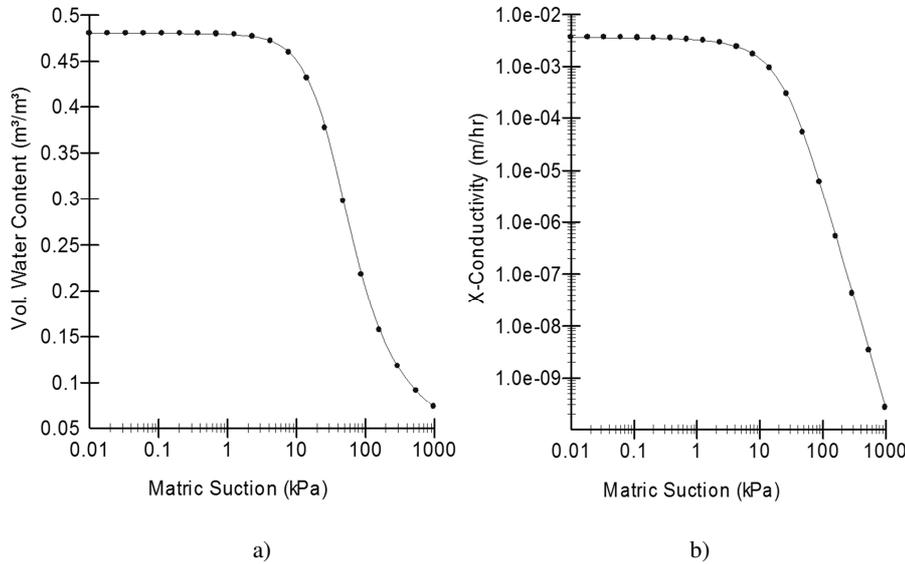


Fig. 2. Graphical representation of a) volumetric water content vs. matric suction;
b) conductivity vs. matric suction

4.2. Methodology

There are many numerical methods and models used to investigate seepage behavior. These common methods are represented by finite difference, boundary elements, finite volume and finite element methods. From all of these methods, finite elements methods are the most powerful and wide range of use relating to different engineering problems, [36]. In this study, seepage and slope stability problems are investigated for two different stages, respectively: steady state and transient condition. For this purposes 'Seep/W' package program that is developed by Geo-Slope International Ltd, which is based on Finite Element Methods (FEM) is used. Concerning to the transient conditions, it consists in two different scenarios, rapid filling, and rapid drawdown. Time of rapid filling is consider 12 hours, while a time of rapid drawdown is consider 156 hours (i.e. 6.5 days). Concerning to the geometrical configuration of dike, (Fig. 3) high of dike is 9 m including foundation, top of dike 5 m, cross-section (width of the dike) is about 45 m and both slopes are 1:2.5. In addition in Fig. 3 the Minimum Water Level (Min.W.L), the Normal Water Level (N.W.L) and the Maximal Water Level (Max.W.L) are shown, respectively. The dike is discretized in triangle mesh with dimension 0.3 m each of them.

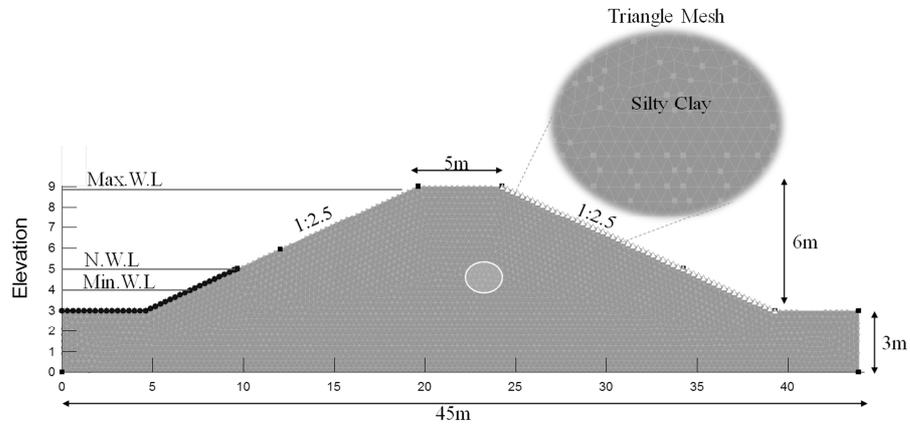


Fig. 3. Geometrical configuration of dike and discretization with triangle mesh elements

4.3. Governing equations

Seepage

Water flowing through the dike in both conditions (saturated/unsaturated), follows the Darcy law Eq. (6).

$$q = k i, \quad (6)$$

where; q is the specific discharge; k is the hydraulic conductivity and i is the hydraulic gradient. While two dimensional seepage flows through the soil for steady state is derived by using Eq. (7) as following:

$$\frac{\partial}{\partial x} \left(k_x \frac{\partial H}{\partial x} \right) + \frac{\partial}{\partial y} \left(k_y \frac{\partial H}{\partial y} \right) + Q = \frac{\partial \theta}{\partial t}, \quad (7)$$

where, H is the total head; k_x is the hydraulic conductivity in the x -direction; k_y is the hydraulic conductivity in the y -direction; Q is the flux; t is the time; and θ is the volumetric water content. From this equation state is noticed that, amount of the rate of flows changes in both directions (x , and y) by taking in account external applied flux is equal with rate of change of the volumetric water content for certain time, [35]. For steady state conditions, amount of water flux entering in certain volume of soil is equal with the escaping water flux, whereas Eq. (7) reduced as below Eq. (8):

$$\frac{\partial}{\partial x} \left(k_x \frac{\partial H}{\partial x} \right) + \frac{\partial}{\partial y} \left(k_y \frac{\partial H}{\partial y} \right) + Q = 0. \quad (8)$$

Concerning to the transient condition, two-dimensional flow through the soil is described as following Eq. (9):

$$\frac{\partial}{\partial x} \left(k_x \frac{\partial H}{\partial x} \right) + \frac{\partial}{\partial y} \left(k_y \frac{\partial H}{\partial y} \right) + Q = m_w \gamma_w \frac{\partial H}{\partial t}, \quad (9)$$

where, m_w is the slope of water content; γ_w is the water content unit weight; k_x is the hydraulic conductivity according to x -direction; k_y is the hydraulic conductivity according to y -direction; Q applied boundary flux; m_s is slope of storage curve; H is the total head and y -direction according y -axis.

According to Galerkin method of residual weighed to the governing differential equation, the finite element equation for two-dimensional seepage can expressed by (10) as following:

$$\tau \int \mathbf{B}^T \mathbf{C} \mathbf{B} dA \mathbf{H} + \tau \int_A \lambda \mathbf{N}^T \mathbf{N} dA \mathbf{H}, t = q \tau \int_L \mathbf{N}^T dL, \quad (10)$$

where, \mathbf{B} is the gradient matrix; \mathbf{C} is the hydraulic conductivity matrix; \mathbf{H} is the nodal vector; \mathbf{N} is the vector of interpolation function; q is the unit flux; τ is the thickness of the mesh element; t is the time; λ is the storage parameter, for case of transient seepage $\lambda = m_w \gamma_w$; A is the designation for summation over the area of given element; and L is the designation for summation over the edge of an element [37]. Since Seep/W package of Geo-Slope International Ltd software, uses the backward differences method so Eq. (10) can be written in simplified form as given Eq. (11):

$$(\Delta t \mathbf{K} + \mathbf{M}) \mathbf{H}_1 = \Delta t \mathbf{Q}_1 + \mathbf{M} \mathbf{H}_0, \quad (11)$$

where \mathbf{K} is the hydraulic conductivity matrix; \mathbf{M} is the water storage matrix; \mathbf{Q}_1 is the nodal flux vector at end of time increment; \mathbf{H}_1 is the head at the end of time increment; \mathbf{H}_0 is the head at the start of time increment and Δt is the time increment.

4.4. Slope stability

As it is mentioned above for both scenarios (i.e. steady state and transient condition), slope stability of dikes is evaluated. Slope/W package based on general limit equilibrium method that include key elements of other methods like Ordinary, Bishop, Janbu and Spencer. In this study, Spencer method is used, which based on two factors of safety equations. These factors of safety can be estimated in two different ways: with respect to moment equilibrium Eq. (12) and horizontal force equilibrium Eq. (13), governing equations are as following:

$$F_m = \frac{\sum [c' \beta R + (N - \mu \beta) R \tan \phi']}{\sum W_x - \sum N_f \pm Dd}, \quad (12)$$

$$F_f = \frac{\sum [c' \beta \cos \alpha + (N - \mu \beta) \tan \phi' \cos \varepsilon]}{\sum W \sin \alpha - D \cos \omega}, \quad (13)$$

while normal force is estimated as following Eq. (14):

$$N = \frac{W + (X_R - X_L) - \frac{c' \beta \sin \alpha + \mu \beta \sin \alpha \tan \phi'}{F}}{\cos \alpha + \frac{\sin \alpha \tan \phi'}{F}}, \quad (14)$$

where, N is the normal force at slice base; X_R is the inter-slice shear force; X_L is the inter-slice shear force on either side of slice; F_m is the factor of safety; W is the slice weight; β is the inclination angel; c' is the soil cohesion; ϕ' is the internal friction angel; α is the inclination of the slice base; D is the concentrated point load and R is the geometric parameter. Dike is analyzed in saturated and unsaturated condition [38], described respectively by Eq. (15) and Eq. (16) as following:

$$S = c' + (\sigma_n - \mu_w) \tan \phi', \quad (15)$$

$$S = c' + (\sigma_n - \mu_\alpha) \tan \phi' + (\mu_\alpha - \mu_w) \tan \phi' \frac{\theta - \theta_r}{\theta_s - \theta_r}, \quad (16)$$

where, S is the degree of saturation, σ_n is the normal stress; μ_α is the dynamic viscosity of material; μ_w is the dynamic viscosity of water; θ_w is the volumetric water content; θ_r is the residual volumetric water content and θ_s is the saturate volumetric water content.

5. Results and discussions

5.1. Seepage analysis

Considering the parameters used as filling materials for construction of the dike, respective analyses are performed. Soil parameters are introduced in the Seep/W model in order to conduct respective seepage analysis. The hydrodynamics process that occurred especially during the rapid filling and drawdown has the significant impact on dike performance. Pore-water pressure and water flux per x -direction is investigated for three different scenarios, (Fig. 4). Regarding to first scenario (i.e. steady state condition), there is noticed a positive pore-water pressure about 35 kPa, that getting

increase up to a distance 12 m according to x -direction, then suddenly is decreasing up to 22 according to x -direction, where negative pore-water pressure about -40 kPa is observed as result of soil unsaturation at this part of dike.

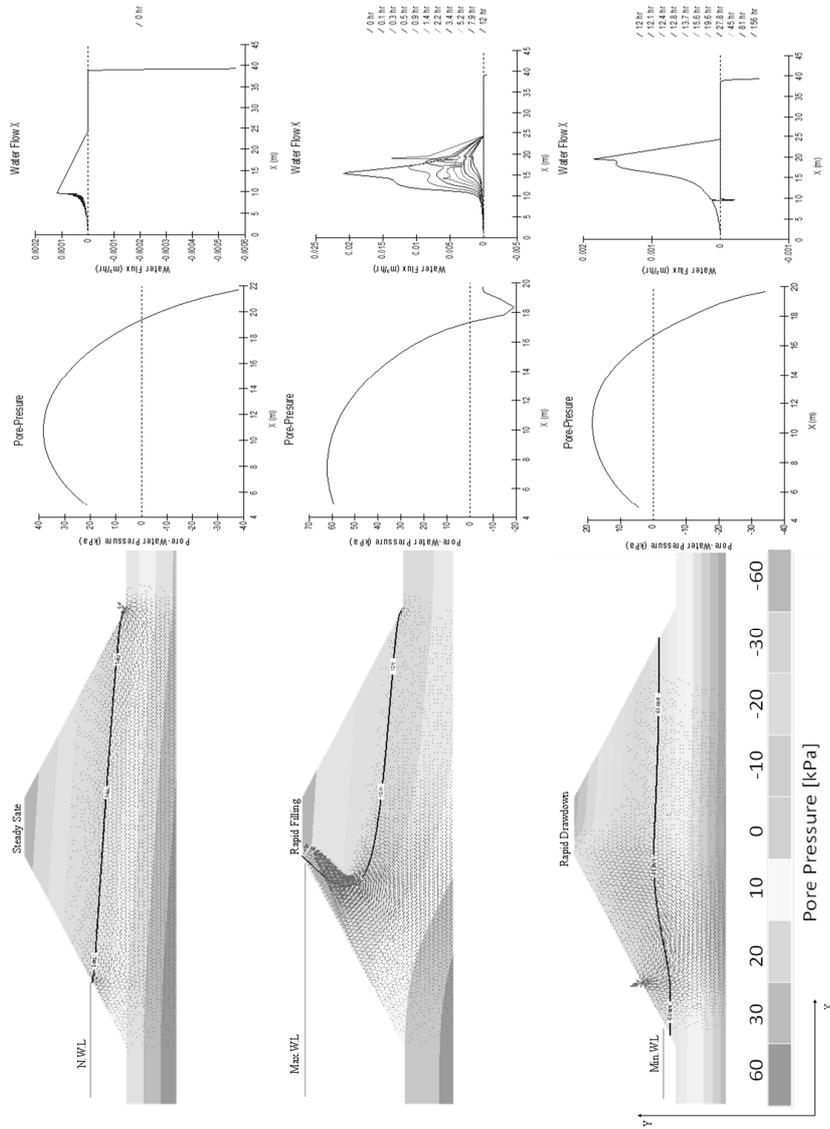


Fig. 4. Seepage analysis, pore-water pressure for three different scenarios: steady state, rapid filling and rapid drawdown

While, maximum water flux about $1.10 \cdot 10^{-4} \text{ m}^3/\text{hr}$ is reached at altitude 8.5 m same distance (12 m) according to x -direction, after that it is decreasing until it becomes 0 at 25 m distance according to x -direction. Concerning to the second scenario (i.e. rapid filling), there is noticed a higher pore-water pressure than the first scenario. Different from the first scenario, in this case maximum pore-water pressure about 60 kPa is reached for shorter distance (i.e. 8 m), then it is decreasing with a trend to a negative pore-water pressure about -15 kPa at 18 m distance according to x -direction. While maximum water flux about $2.10 \cdot 10^{-4} \text{ m}^3/\text{hr}$ is reached at distance about 17 m after 12 hours, afterwards is decreasing until become zero at distance 25 m according to x -direction, fluctuation of water flux for respective time is presented graphically (Fig. 4), rapid filling. During the rapid drawdown, compare with the two aforementioned scenarios, it is noticed significant reduction of pore-water pressure; whereas maximum value of pore-water pressure about 15 kPa is reached at a distance 11 m, while drastic reduction and negative pore-water pressure is occurred as result of water withdraw from the soils and unsaturation process. Concerning to the water flux is decreasing drastically, until it becomes 0 at distance 11 m after 156 hr of water withdraw at the inner part of dike structure. Detailed representation of water flux is shown at a respective graph (Fig. 4), rapid drawdown. Fluctuation of pore-water pressure and water flux according to x -direction for the certain time is effected from soil parameter (i.e. conductivity) and hydrodynamic processes that happening especially inside of the dike structure. Concerning to last scenarios, during rapid filling cohesion of filling material almost disappear so in these condition positive pore-water pressure is occurred while during rapid drawdown as result withdraw of water from soil particles is noticed decrease and afterwards negative pore-water pressure is occurred, (Fig. 5).

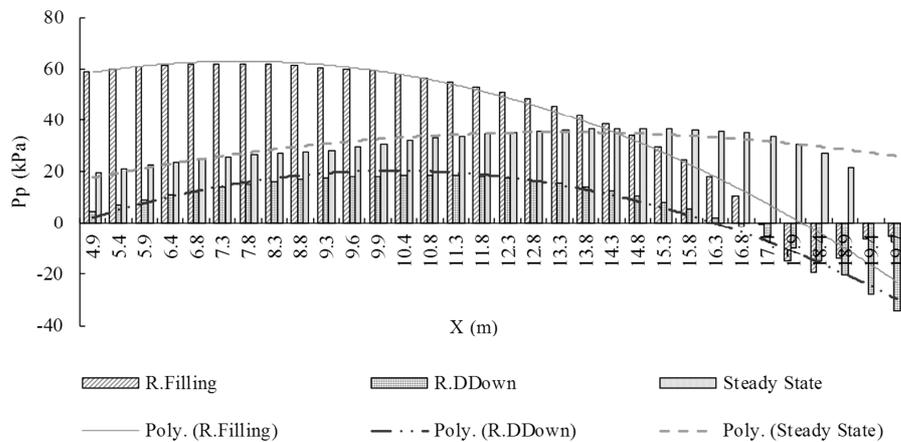


Fig. 5. Comparison of pore-water pressure occurred during steady state, rapid filling and rapid drawdown fitted with respective aforementioned polynomial functions

In addition there the correlation between pore pressure and prolongation distance of water flux is estimated along to x -direction. In all cases good correlation with a determination coefficient $R^2 > 0.8$ is noticed. The correlation between pore pressure and

prolongation distance is expressed through following polynomial equations, Eqs. (17), (18), (19):

$$P_p = -0.0544x^2 + 2.0625x + 15.631, \quad R^2 = 0.8495 \quad (\text{SteadyState}), \quad (17)$$

$$P_p = -0.1343x^2 + 1.7934x + 57.099, \quad R^2 = 0.9539 \quad (\text{RapidFilling}), \quad (18)$$

$$P_p = -0.1337x^2 + 3.3814x - 1.0106, \quad R^2 = 0.9848 \quad (\text{RapidDraw Down}). \quad (19)$$

If during the rapid filling dike structure maintains its stability as result of hydrostatic pressure, on the other hand during rapid drawdown, dike structure is likely to fail its stability as result of seepage process which induce migration of soil particles outside the dike, [39].

5.2. Slope stability analysis

After seepage analysis, slope stability analysis is performed for each respective scenario. During the slope stability analysis, shear strength of soils and factor of safety is observed, (*Fig. 6*). Concerning to the first scenario (i.e. steady state condition), the factor of safety is about 2.45; it remain constant. While shear strength is getting increase about 36 kPa up to 16 m distance according to x -direction, after that it is decreasing suddenly as the result of less consolidation of the outer side of the dike.

Regarding stability analysis for the second scenario (i.e. rapid filling); there is a significant increase related to the factor of safety especially during the first hours of rapid filling. Afterward, there is reached an equilibrium that corresponds to the factor of safety about 5.7. The sudden increase of the factor of safety is due to the temporal compaction of soils by the hydrostatic pressure. While shear strength is increasing up to the distance about 18 m then it is suddenly decreasing as the result of the reduction of soils strength parameter, in addition, shear strength in this scenario is lower than the first scenario. Concerning to the third scenario (i.e. rapid drawdown), it is noticed a significant decrease of a factor of safety comparing with the first and second scenario as well. Differences of safety factors regarding third scenario are about 1.2% lower than first scenario and 57% lower than a second scenario. While concerning the shear strength it is increasing about 33 kPa up to 13 m distance according to x -direction, after that it is decreasing suddenly.

In addition, shear strength comparing with the second scenario represent higher value while comparing with the first scenario there is the significant reduction, (*Fig. 7*).

So, rapid change of water level at dikes structure generally lead to the degradation and reduction of soil parameters, in this condition there is a higher threat of structure failure. The correlation between shear strength and prolongation distance of the water flux along x -direction is estimated. In all cases is noticed good correlation with a determination coefficient $R^2 > 0.8$.

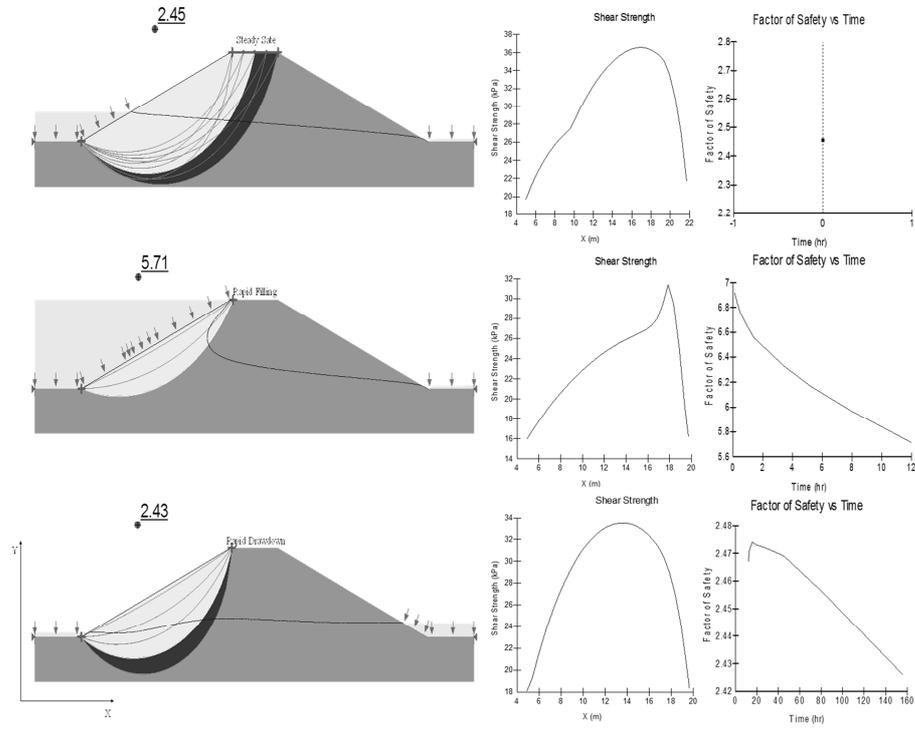


Fig. 6. Slope stability analysis, factor of safety and shear strength for three different scenarios: steady state, rapid filling and rapid drawdown

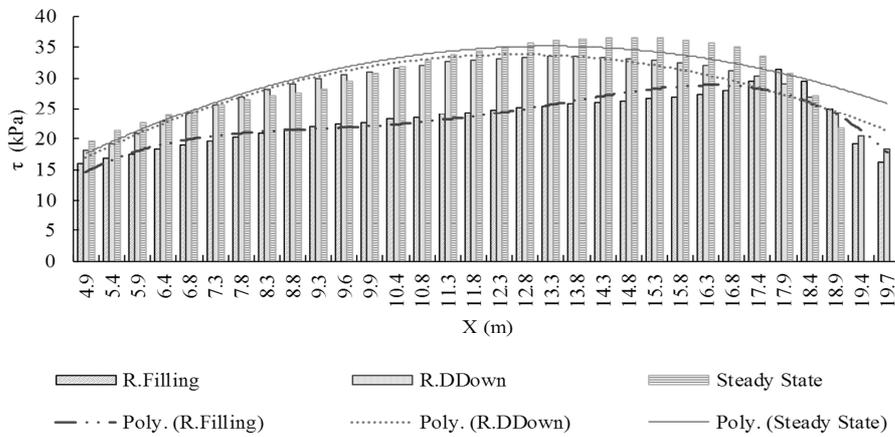


Fig. 7. Comparison of shear strength for three scenarios: steady state, rapid filling and rapid drawdown fitted with respective aforementioned polynomial functions

The correlation between shear strength and prolongation distance i expressed through following polynomial equations, Eqs. (20), (21), (22):

$$\tau = -0.0544x^2 + 2.0625x + 15.631, \quad R^2 = 0.8495 \quad (\text{SteadyState}), \quad (20)$$

$$\tau = -0.0003x^4 + 0.0162x^3 - 0.3159x^2 + 2.7574x + 12.201, \quad (21)$$

$$R^2 = 0.8869 \quad (\text{Rapid Filling}),$$

$$\tau = -0.0606x^2 + 2.1403x + 14.937, \quad R^2 = 0.9539 \quad (\text{RapidDrow Down}). \quad (22)$$

6. Conclusions

In this paper seepage process and slope stability of dike, structure is investigated through numerical modeling for different scenarios. Scenarios considered during numerical modeling are as following: steady state, rapid filling and rapid drawdown. The investigation of seepage process during the first scenario revealed that there is certain water volume passing through the dike structure. In this case, there is no any threat concerning the stability of dike, even that presence of water inside the dike structure lead to a reduction of mechanical properties of soil. For second scenario, safety factor is increased significantly up to 5.71 but the reduction of shear strength indicates that increase of dike stability is just temporary phenomenon's that happen as result of temporary compaction of the filling materials by hydrostatic forces. Whereas during the third scenario; stability of the dike structure is decreased compared with the two first scenarios, safety factor achieved during this scenario is 2.43. The decreasing of the factor of safety is revealed perceptibly by the significant reduction of soils parameters like; shear strength as presented on the respective graph. In this case study is noticed that during rapid filling Finally after detailed investigation of the seepage phenomenon through the dike structure, it is conclude that internal and external hydrodynamic processes and especially seepage is a serious threat that should be considered not just during design process but also operation, even that materials used for construction of the dike structure or earth structures in generally may have very good parameters. In addition, designers should be aware what kind of filling materials should be used for construction of earth structures and adaptation measures in case of structure failure. Especially for high earth structures, using silt-clay as filling materials, based on standards recommended may lead to the rapture of structure and then catastrophic disaster. Also, proper riprap layers should be considering during the design phase in order to increase the safety of dike structure and to prevent any possible failure.

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